

Lateral Stability

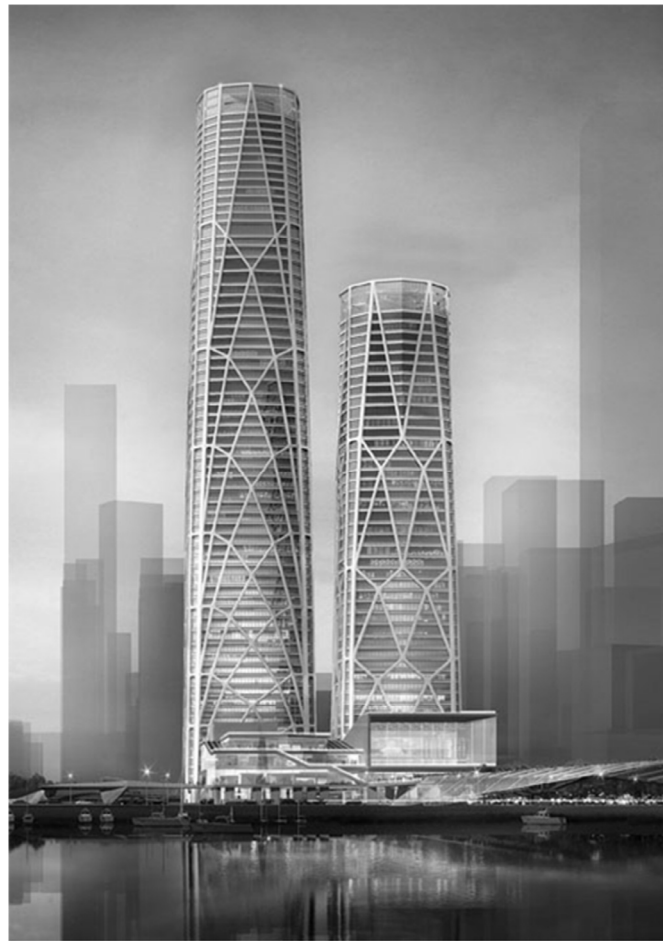
Lateral Loads

Frame Bracing

Shearwalls

Diaphragms

Bracing Configurations



CITIC Financial Center
Shenzhen, China
SOM

Load Combinations

Load Types

- Dead Load - D
- Roof Live Load - L_r
- Floor Live Load - L
- Snow Load - S
- Wind Load - W
- Earthquake - E

Allowable Stress Design (ASD)

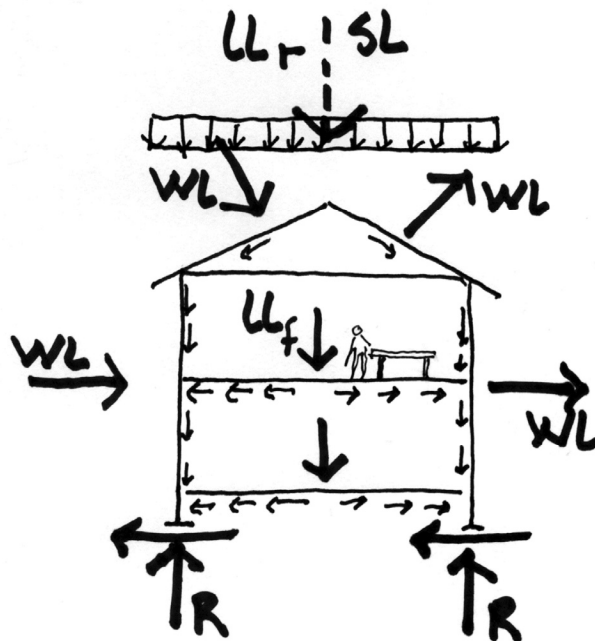
Not factored

- D
- D + L
- D + (L_r or S)
- D + 0.75L + 0.75(L_r or S)
- D + (0.6W)
- D + 0.75L + 0.75(0.6W) + 0.75(L_r or S)
- D + 0.7E_v + 0.7E_h

Strength Design (LRFD)

With gamma (γ) safety factors

- 1.4 D
- 1.2 D + 1.6 L_r + 0.5(L_r or S)
- 1.2 D + 1.6(L_r or S) + (L or 0.5W)
- 1.2 D + 1.0W + L + 0.5(L_r or S)
- 0.9D + 1.0W
- 1.2D + E_v + E_h + L + 0.2S
- 0.9D - E_v + E_h



Load Paths

Vertical Loads

gravity
D, L, Lr, S,

Lateral Loads

wind
seismic

FIGURE 1
VERTICAL LOAD PATH

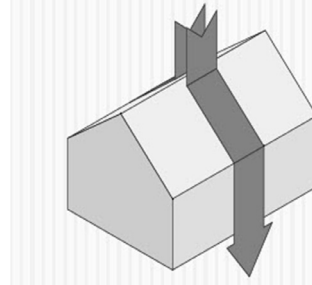
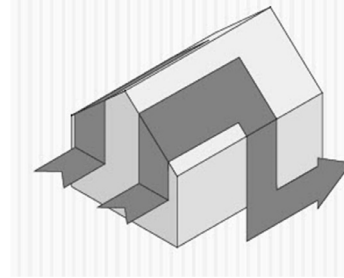
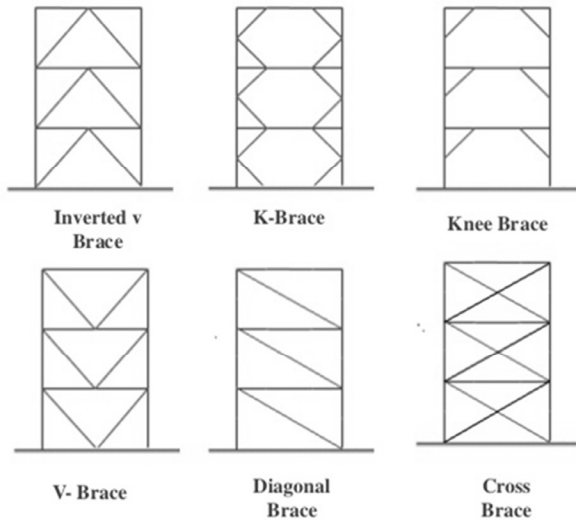


FIGURE 2
LATERAL LOAD PATH



Frame Bracing



John Hancock Tower, Chicago
SOM, 1968
Engineer: Fazlur Khan

Lateral Frame Bracing



Lateral Bracing
tension and compression
(Michigan North Quad)



Diagonal Tension Counters (X-Bracing)
(Buck Steel Buildings)

Lateral Stability

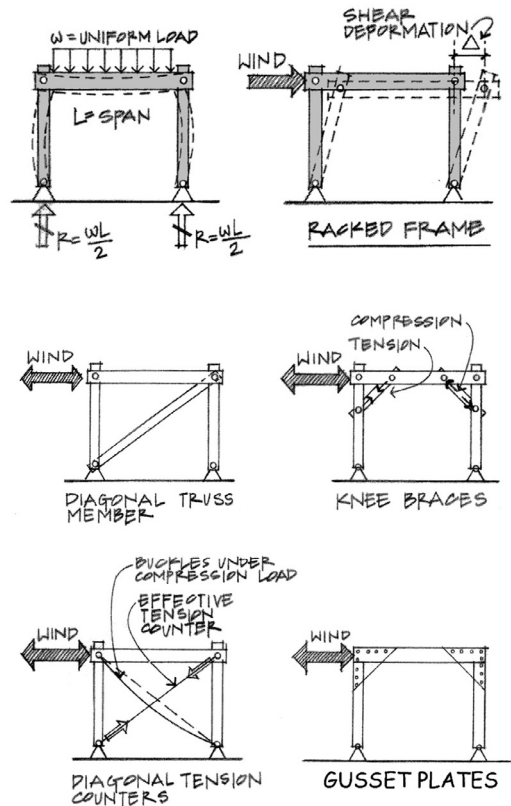
A system needs to be stable in all directions – x,y, and z.

Dead , Live and Snow Loads are vertical due to gravity.

Wind and Seismic Loads are primarily horizontal or lateral.

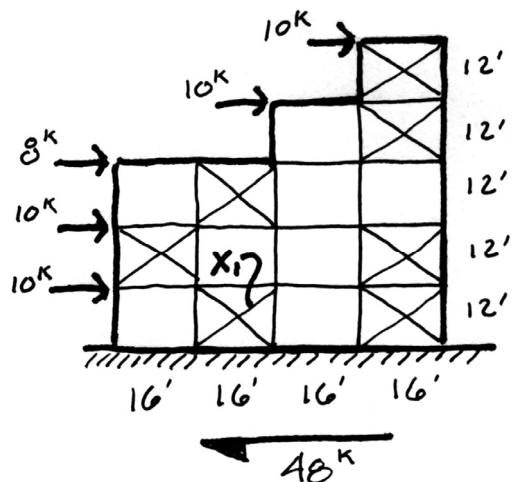
Lateral bracing can be achieved with:

- Diagonal truss member
- X-bracing members
- Knee bracing
- Gusset plates



Example Frame Bracing

- Check for stability. At least one rigid frame per story
- Convert distributed loads to point loads acting at floors.
- Solve the horizontal reaction for the whole system.
- Assume the bracing carries tension only



Base shear = 48k

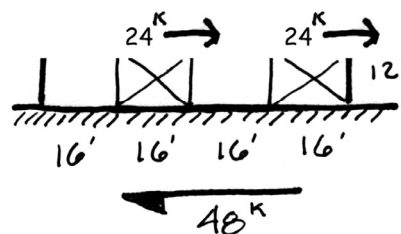
$$\sum F_H = 0$$

$$0 = 10 + 10 + 8 + 10 + 10 - R$$

$$R = 48^k$$

Example Frame Bracing cont.

- Cut a FBD horizontally through the story containing the brace being solved.
- Sum horizontal forces to find the horizontal component in the braces. Assume load is divided evenly among braces in a story.
- In this case only the tension bracing carries load (rods or cables)
- Find the vertical component by proportions or trig function



$$\sum F_H = 0$$

$$0 = -48 + H_1 + H_2$$

$$H_1 = H_2 = 24^k$$

$$\frac{12}{16} = \frac{V}{24}$$

$$V = 18$$

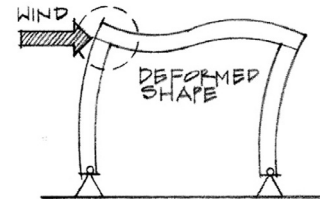
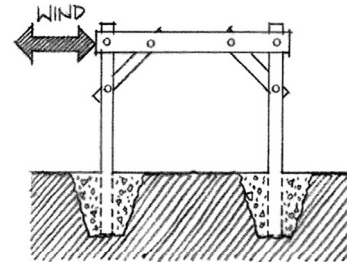
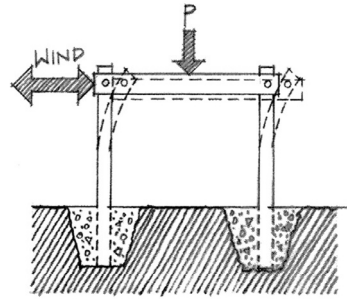
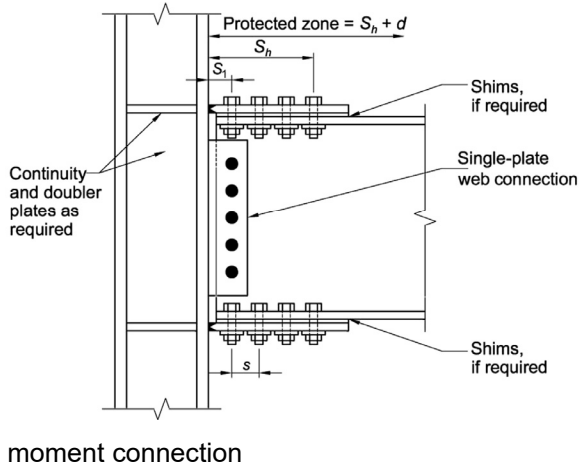
$$X_1 = \sqrt{18^2 + 24^2} = 30^k$$

Lateral Stability

A system needs to be stable in all directions – x,y, and z.

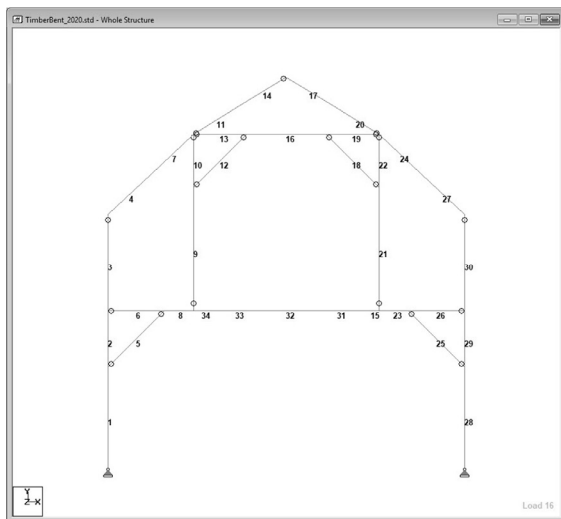
Fixed (moment) connections in a rigid frame can also provide stability.

In a fixed frame the members act in both compression and bending.



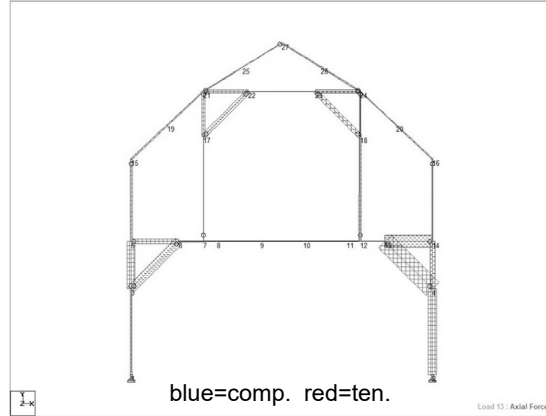
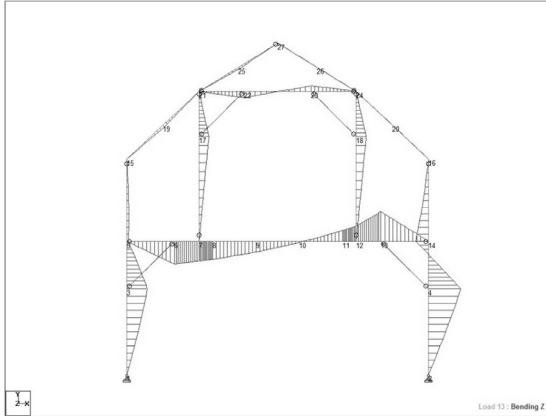
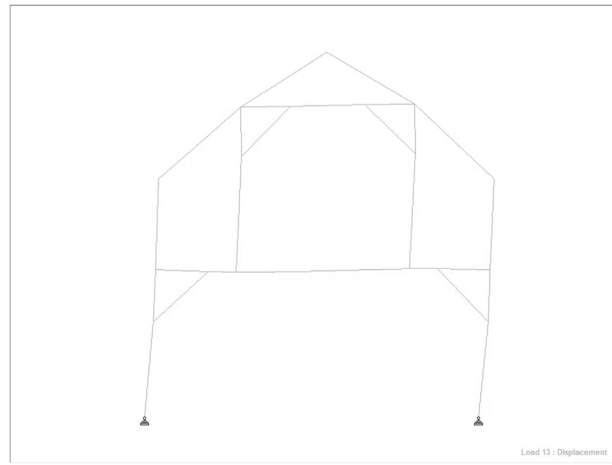
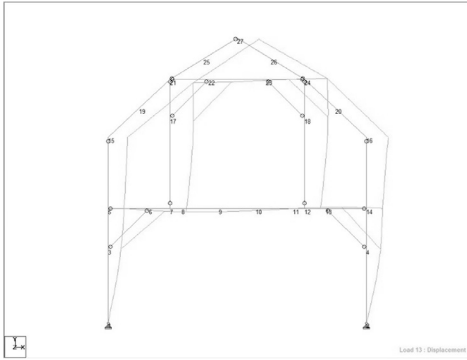
Timber Frame Bracing

John Pariseau's Timber Frame
Load Case: $D + 0.6W$



Timber Frame Bracing

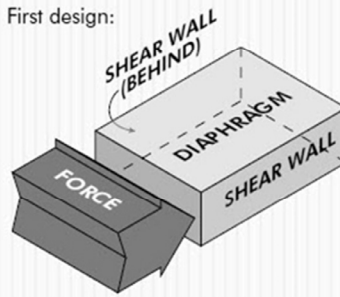
John Pariseau's Timber Frame



Diaphragms and Shear Walls

LATERAL LOAD ANALYSIS MUST BE CONDUCTED ALONG BOTH AXES OF STRUCTURE

First design:



and then design:

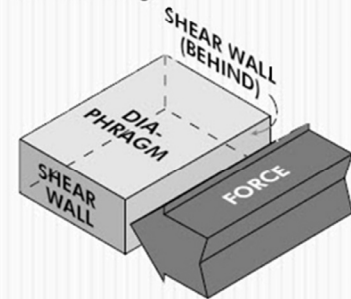


FIGURE 4

SEISMIC FORCES ACTING ON MASS

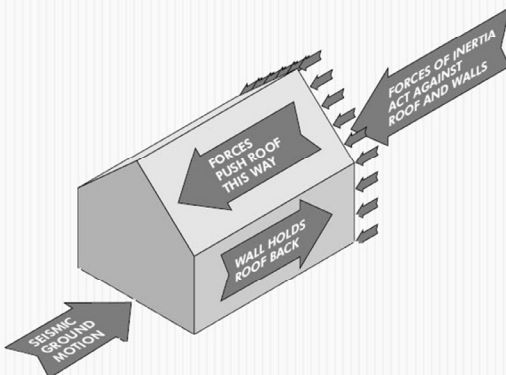
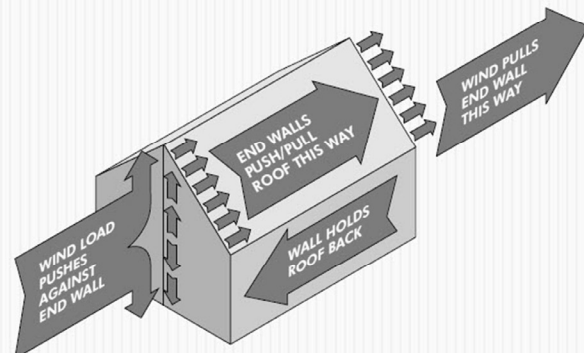


FIGURE 5

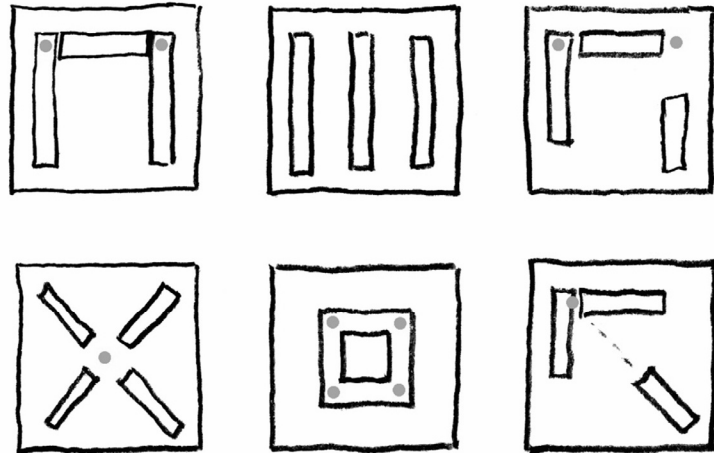
WIND FORCES ACTING ON AREA



Lateral Force Resistance

Stability requires at least 2 points of intersection.

Force is more evenly resisted with centroid of walls in the kern of slab



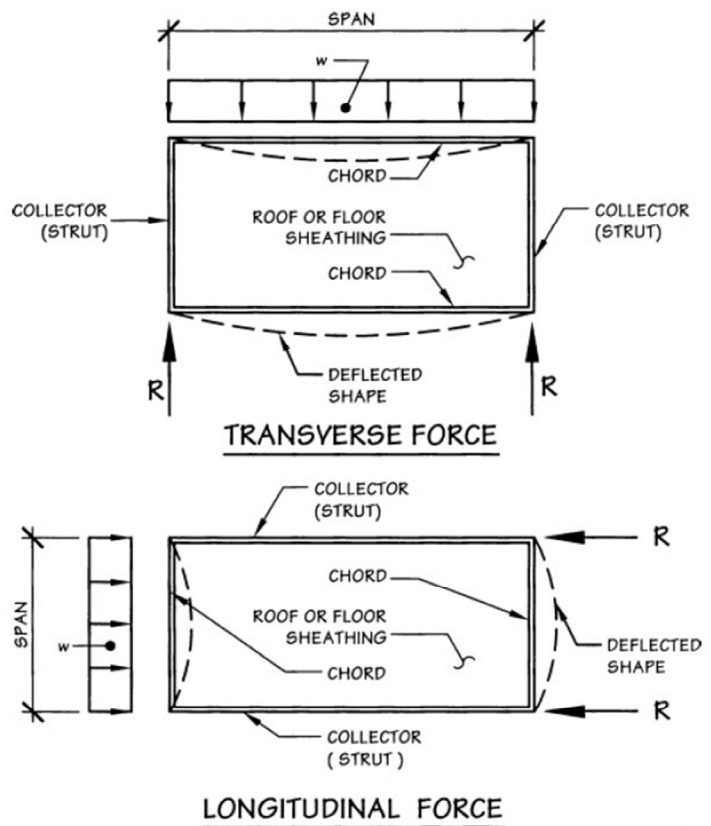
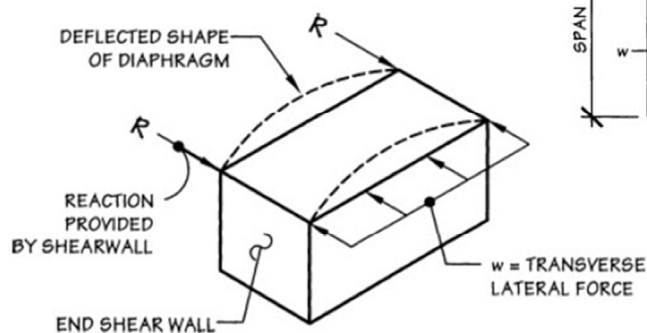
Definitions

Diaphragm – a flat structure which acts as a deep beam to resist in plane loads.

Shear Wall – a vertical structure which acts as a cantilevered diaphragm

Chord – the edge member of a diaphragm

Collector (strut) – transfers the force from the diaphragm to the shear wall



Shear Wall Design Elements

- Panel Thickness
- Panel Grade
- Nail spacing
- Base shear anchors
- Hold down anchors (at ends of each wall)
- Placement for lateral stability
- Fastening at edges (chords)

A Shear Wall...	A Diaphragm...
Is vertical	Is horizontal (or nearly so)
Is designed like a cantilevered beam	Is designed as a simply supported beam
Table has only blocked values, because a shear wall is always blocked*	Table has both blocked and unblocked diaphragm values

*A code requirement.

FIGURE 11
SHEAR WALL SEGMENT

Local building codes typically stipulate a minimum w of $h/3.5$

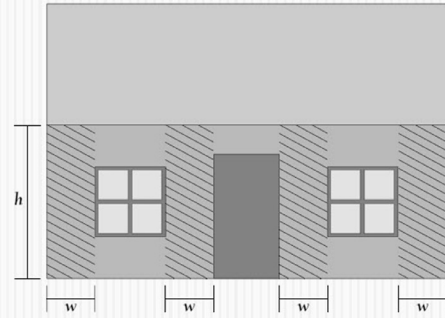


FIGURE 13
OVERTURNING

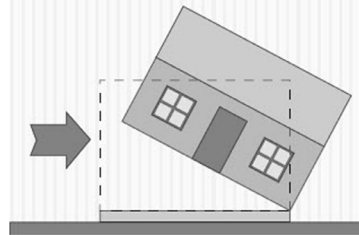


FIGURE 12
BASE SHEAR

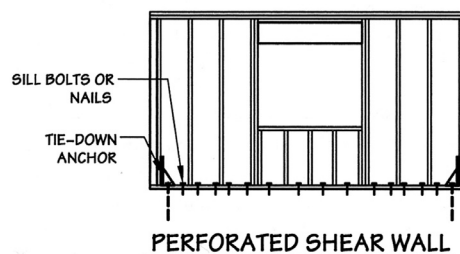
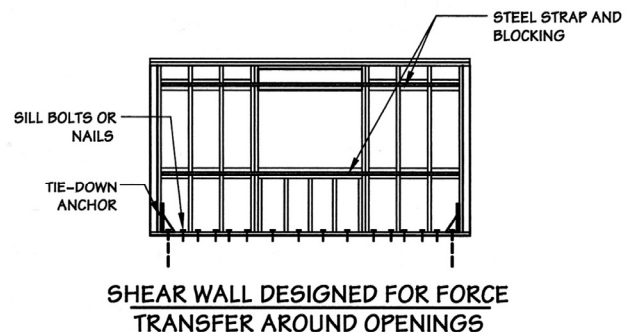
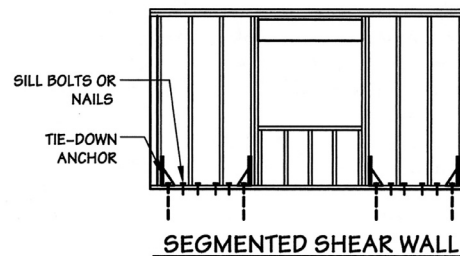
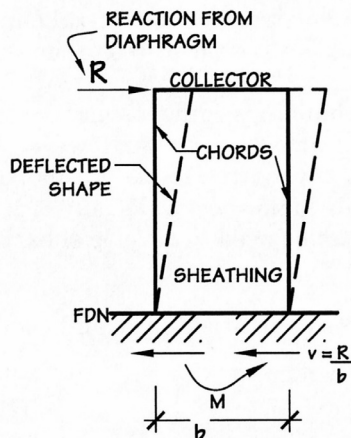


Three Shear Wall Types

(used in light framing)

Design considerations:

- Sheathing – type and thickness
- Sheathing nailing – size and spacing
- Chord design – tension and compression
- Collector design – tension and comp.
- Anchorage – hold-downs, shear ties
- Shear panel proportions – $h:w$
- Deflection



Shear Wall Types

Acts like a vertical cantilever beam

Let-in Wall Bracing – 45° - limited to single or top story

Wall Board – requires 8 ft length

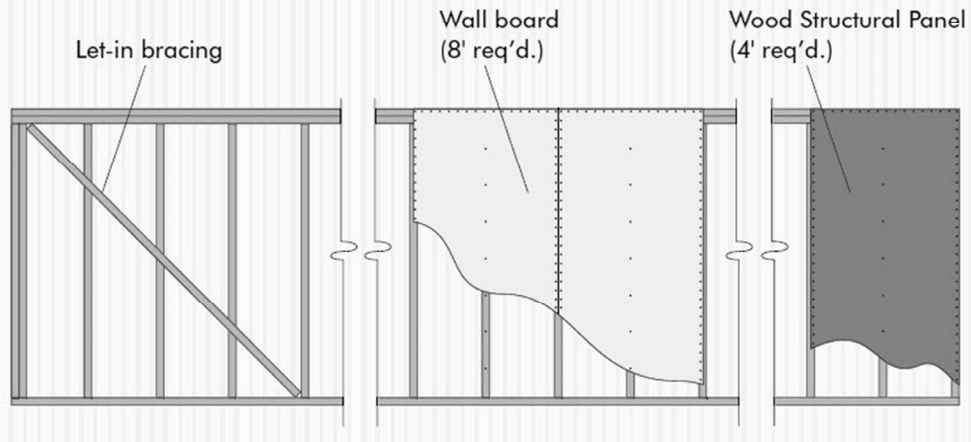
Wood Structural Panel – requires 4 ft length – 3 times stronger by length

**TABLE 2305.3.4
MAXIMUM SHEAR WALL DIMENSION RATIOS**

TYPE	MAXIMUM HEIGHT-WIDTH RATIO
Wood structural panels or particleboard, nailed edges	For other than seismic: 3 ¹ / ₂ :1 For seismic: 2:1 ^a
Diagonal sheathing, single	2:1
Fiberboard	1 ¹ / ₂ :1
Gypsum board, gypsum lath, cement plaster	1 ¹ / ₂ :1 ^b

a. For design to resist seismic forces, shear wall height-width ratios greater than 2:1, but not exceeding 3¹/₂:1, are permitted provided the allowable shear values in Table 2306.4.1 are multiplied by 2w/h.
b. Ratio shown is for unblocked construction. Height-to-width ratio is permitted to be 2:1 where the wall is installed as blocked construction in accordance with Section 2306.4.5.1.2.

SHEAR RESISTING ELEMENTS PRESCRIPTIVE CORNER BRACING/WALL BRACING



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Shear Wall Connections

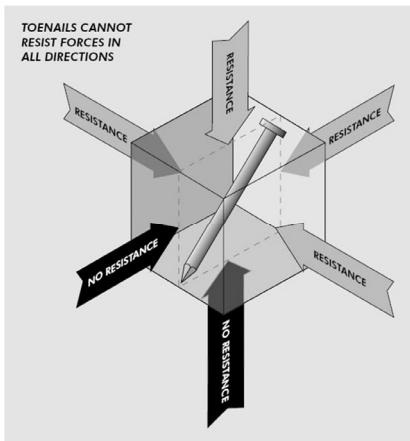
Connections need to transmit force in 6 directions (3 axes)

Toenails – not adequate

Hold-down Anchors

Base Shear Anchors

ENGINEERED SHEAR WALLS

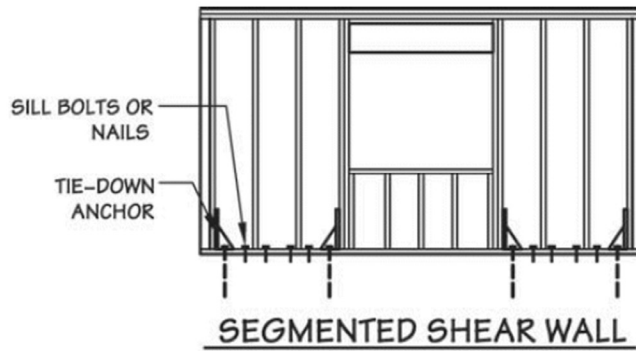
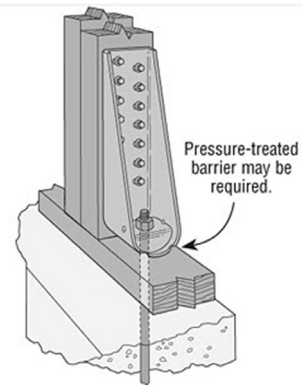
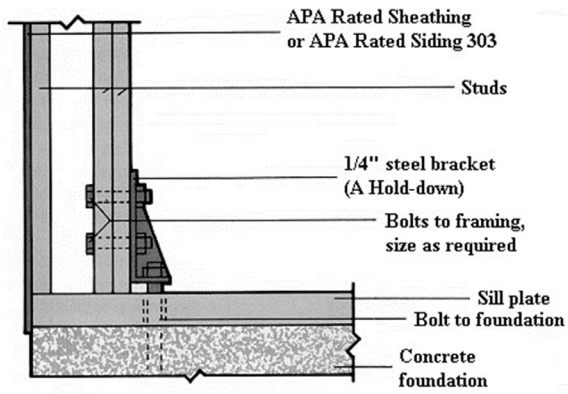


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Anchors and Tie-downs



Multi-story shear walls

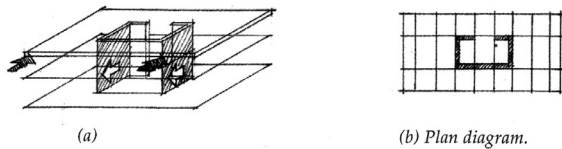


Figure 4.55 Shearwalls at the central circulation core.

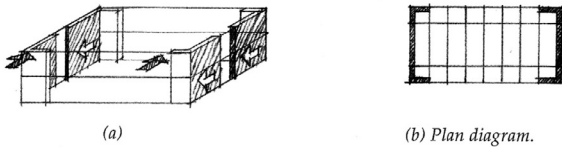


Figure 4.56 Shearwalls at the exterior corners.

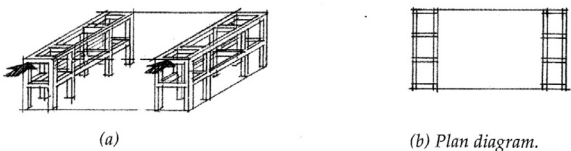


Figure 4.57 Rigid frames at end bays (can also comprise the entire skeleton).



Brock Commons Tallwood House
University of British Columbia, Vancouver, Canada