ARCHITECTURE 314 STRUCTURES I

Equilibrium of Rigid Bodies

- Equilibrium
- · Parallel Force Resultant
- Load Distribution
- External Reactions



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Newton's First Law

An object at rest will remain at rest unless acted upon by an outside, external net force.

$$\sum \mathbf{F}_{x} = 0 \quad \sum \mathbf{F}_{y} = 0 \quad \sum \mathbf{M} = 0$$

Horizontal Equilibrium

$$\sum \mathbf{F}_{x} = 0$$

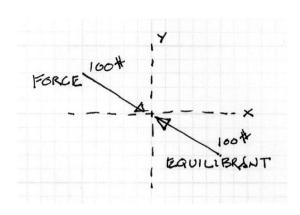
Vertical Equilibrium

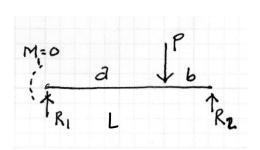
$$\sum \mathbf{F}_{y} = 0 = \mathbf{R}_{1} + \mathbf{R}_{2} - \mathbf{P}$$
$$\mathbf{R}_{1} + \mathbf{R}_{2} = \mathbf{P}$$

Rotational Equilibrium

$$\sum \mathbf{M}_1 = 0 = \mathbf{P}\mathbf{a} - \mathbf{R}_2 \mathbf{L}$$

$$\mathbf{R}_2 = \frac{\mathbf{P}\mathbf{a}}{\mathbf{L}}$$





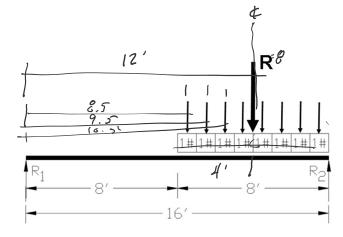
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Parallel Force Resultant

The resultant is a single force that has the same effect as a group of forces.

The resultant is located at the center or *centroid* of the group of forces.



$$\sum (\mathbf{F} \times d) = \mathbf{R} \times \overline{d}$$

$$\mathbf{R} = \sum \mathbf{F}$$

$$\overline{d} = \frac{\sum (\mathbf{F} \times d)}{\sum \mathbf{F}}$$

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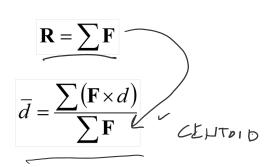
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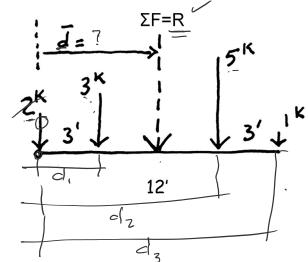
Parallel Force Resultant

The resultant is a single force that has the same effect as a group of forces.

Since the resultant is equivalent to the group of forces, it can be used in place of the group.

$$\sum (\mathbf{F} \times \mathbf{d}) = \mathbf{R} \times \overline{\mathbf{d}}$$



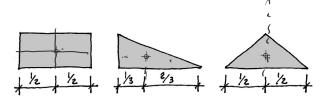


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Center of Area (centroid)

In determining external reactions, the total load can be represented as a single (resultant) load at the center of gravity. In 2 dimensions this is the center of area or the centroid.

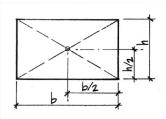


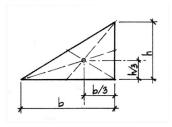
Centroids:

rectangles = midpoint

triangles = 1/3 point

symmetric = center





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Load Distribution through the Centroid

Self Load

Through center of gravity

Uniform Load

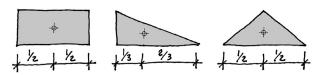
Constant over length examples:

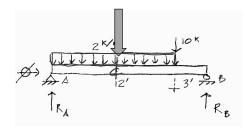
beam selfweight rectangular floor system

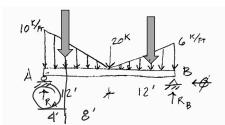
Uniformly Varying Load

Linear change over length examples:

snow drifts
fluid pressure
triangular floor areas



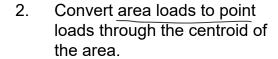




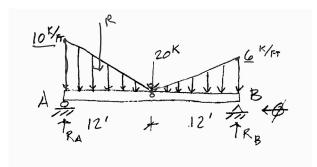
Equilibrium of Forces

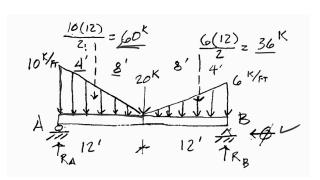
Example 1: Beam End Reactions

Label components of reactions.
 Depending on the support condition, include vertical, horizontal and rotational.



3. Since there is only one horizontal force, it must equal zero.



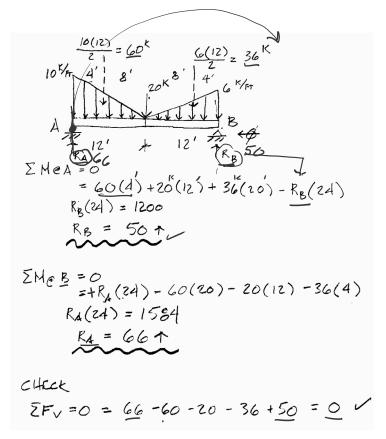


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Equilibrium of Forces

Example: Beam End Reactions

- Use the summation of moments about A to find R_B.
- 5. Use the summation of moments about B to find R_A .
- 6. Check calculation by summing vertical forces.



Ridged Body Supports

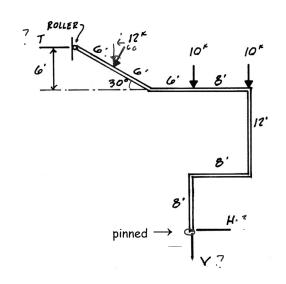
Example 2

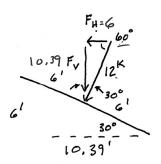
- Label components of reactions. Depending on the support condition, include vertical, horizontal and rotational.
- 2. Convert all point loads into x and y components.

$$F_{H} = 5 \text{ in 30 (12)} = G^{K}$$

or

 $G' : F_{H} = G^{K}$
 $F_{H} = G^{K}$
 $F_{V} = \sqrt{12^{2} - G^{2}} = 10.39^{K}$





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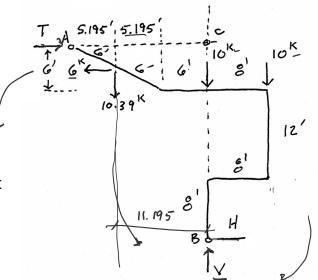
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Ridged Body Supports

Example 2

- 3. Since there is only one unknown vertical force (V), find that first.
- 4. Use the summation of moments about B to find T.



$$\sum F_{v} = 0 = -10.39^{k} - 10^{K} - 10^{K} + 4$$

$$V = 30.39^{k} \uparrow$$

$$EM_{CB} = 0 = T(26') - G^{k}(23') - 10.39^{k}(11.195') + 10^{k}(3')$$

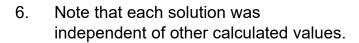
$$T(26') = 138^{k-1} + 116.3^{k-1} - 80^{K-1} = 174.3^{k-1}$$

$$T = 6.70^{k} \rightarrow$$

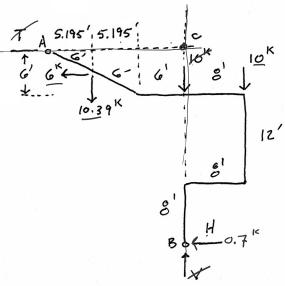
Ridged Body Supports

Example 2

5. Use the summation of moments about C to find H.



7. Finally check calculations by summing horizontal forces. They should balance out to zero.



EMec=0=
$$6^{\kappa}(3')$$
-10.39 $^{\kappa}(11.195')$ +10 $^{\kappa}(8')$ + $\frac{\epsilon}{11}(26')$
 $11(26') = -18^{\kappa-1}$ +116.3 $^{\kappa-1}$ -80 $^{\kappa-1}$ = 18.3 $^{\kappa-1}$
 $11 = 0.70^{\kappa}$

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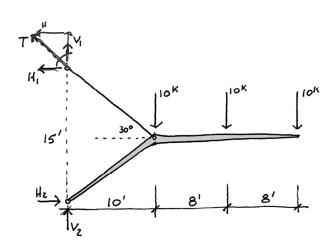
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Cantilever Frame

Example 3

Find the reactions of the cable supported frame.

Hint: $V_1/H_1 = \text{Tan } 30^\circ$



Cantilever Frame

Example 3

Find the reactions of the cable supported frame.

$$\sum_{H_1 = 540/15} = \frac{36^{K}}{10(10) + 10(18) + 10(26)}$$

$$H_1 = \frac{540/15}{15} = \frac{36^{K}}{10(10) + 10(18) + 10(26)}$$

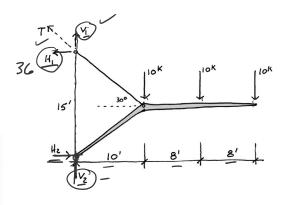
$$\tan 30^{\circ} = 0.\frac{57735}{10(10)} = \frac{\sqrt{1}}{10(10)}$$

$$V_1 = \frac{20.78^{K}}{10(10)}$$

$$\sum_{V_2 = 9.22^{K}} \uparrow$$

$$\sum_{V_3 = 0} F_{14} = 0 = H_2 - 36$$

$$H_2 = 36^{K} \rightarrow V$$



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Other Examples

