Architecture 314
Structures I

Plane Trusses Method of Joints

Definition and Assumptions
Nomenclature
Stability and Determinacy
Analysis by joints



Phaeodaria – Ernst Haeckel

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Definitions and Assumtions of Truss Systems

2 Force Members

Pinned Joints <

Concurrent Member Centroids at Joints

Joint Loaded <

Straight Members ~

Small Deflections



Bullring Covering, Xàtiva, Spain Kawaguchi and Engineers, 2007 Slide 1 of 19

Nomenclature

Panels

· Segments: left to right

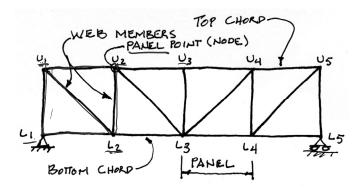
Joints

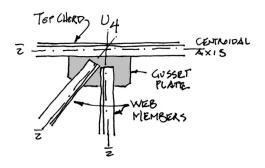
• Upper: U1, U2, U3...

• Lower: L1, L2, L3...

Members

- Chords
- Web





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Trussed Force Systems

2D Trusses

Concurrent Coplanar

3D Trusses

· Concurrent Non-Coplanar



Foster Bridge, 1889 Ann Arbor, Michigan



University of Michigan Architectural Research Lab Unistrut System, Charles W. Attwood

Stability and Determinacy of 2D Trusses

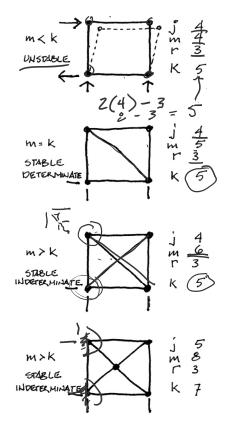
For:

- j joints
- · m members
- r reactions (restraints)

$$\widehat{(k)} = 2j - r$$

Three conditions

- m < k unstable
- m = k stable and determinate
 m > k stable and indeterminate



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Quiz

For each of the following trusses, determine whether they are:



- A) Stable
- B) Unstable

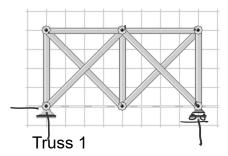
$$k = 2j - r$$

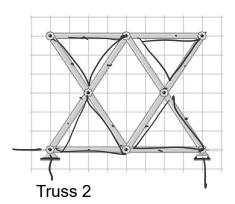
12 13

- m < k unstable
- m = k stable and determinate
- m > k stable and indeterminate

$$2(8) - 3 = 13$$

m = 12

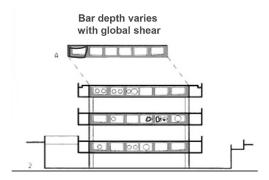




Vierendeel "Truss"

Not a true truss

Moment frame structure Rigid joints as moment connections Flexure in members



Salk Institute, La Jolla. Architect: Louis Kahn

Engineer: Komendant and Dubin



Vierendeel bridge at Grammene, Belgium Photo by Karel Roose

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Truss Analysis

Method of Joints

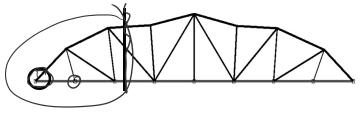
Method of Sections

Graphic Methods

James Clerk Maxwell 1869 M. Williot 1877 Otto Mohr 1887 Heinrich Müller-Breslau 1904 William Baker, SOM

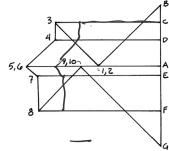
Computer Programs

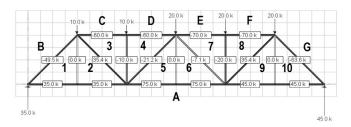
Dr. Frame (2D) STAAD Pro (2D or 3D)
West Point Bridge Designer





James Clerk Maxwell

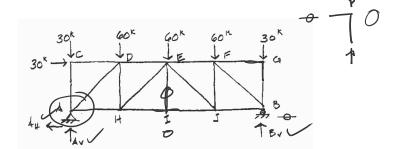


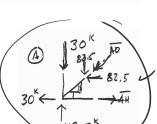


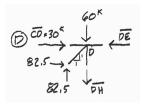
Method of Joints - procedure

- 1. Solve reactions (all external forces)
- 2. Inspect for zero force members (T's & L's)
- 3. Cut FBD of one joint
- LL
- 4. Show forces as orthogonal components
- 5. Solve with $\Sigma {\rm F_H}$ and $\Sigma {\rm F_V}$ (no $\Sigma {\rm M})$
- 6. Find <u>resultant member</u> forces (Pythagorean Formula)









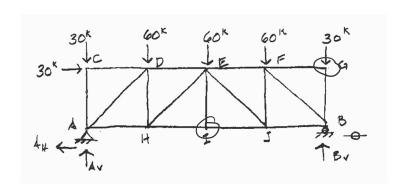
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Inspection of Zero Force Members

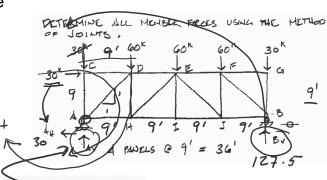
T - jointsL - joints



$$\begin{array}{c|c} 135^{k} & \phi & \overline{1}\overline{1} \\ \hline & & & \\ \hline & & \\ \hline \end{array}$$

$$\begin{array}{c}
\overline{4} & \overline{4} & \overline{4} & \overline{4} & \overline{4} \\
\uparrow & \overline{4} & \overline{4} & \overline{4} & \overline{4} & \overline{4} & \overline{4} \\
\uparrow & \overline{4} \\
\hline
\end{array}$$

1. Solve the external reactions for the whole truss.

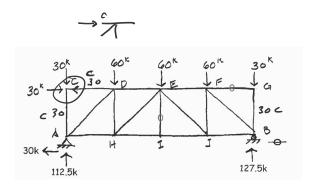


PREACTIONS: $\Sigma F_{H} = 0 = 30 - A_{H}$ $A_{H} = 30^{k} \leftarrow$ $\Sigma HeA = 0 = 30(9) + 60(9) + 60(12) + 60(27)$ $+30(36) - B_{V}(36)$ $B_{V}(36) = 4590$ $B_{V} = 127.5 \times 1$ $\Sigma F_{V} = 0 = A_{V} + 127.5 - 2(30) - 3(60) = 0$ $A_{V} = 112.5 \times 1$

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Method of Joints - example

- 2. T or L joints by inspection.
- 3. Cut FBD of joint
- 4. Show orthogonal components
- 5. Solve by ΣF horz. and vert.

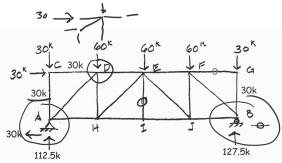


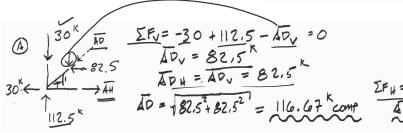
G
$$\overrightarrow{F_4}$$
 0 $\Sigma F_H = 0 = F_4 (ZEFO)$

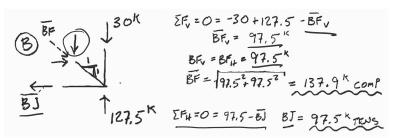
C $\Sigma F_V = 0 = -30^K + B4$
 $\overrightarrow{B_4}$ $\overrightarrow{B_4} = 30^K COMP$

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Continue with joints having only one unknown in either horizontal or vertical direction. Generally work starting at the reactions.







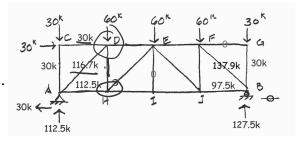
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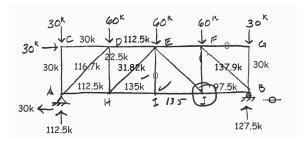
Method of Joints - example

Continue moving across the truss, joint by joint. Solve by ΣF_H and ΣF_V .

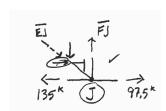


$$\Sigma F_{V} = 0 = 22.5 - EHV$$
 $EH_{V} = 22.5$
 $EH_{V} = EH_{H} = 22.5$
 $EH_{V} = EH_{H} = 22.5$
 $EH_{V} = 12.5^{2} + 22.5^{2} = 31.82$
 $EH_{V} = 13.5 \times 10.5$
 $EH_{V} = 13.5 \times 10.5$

Continue moving across the truss, joint by joint. Choose joints that have only one unknown in each direction, horizontal or vertical.



$$\begin{array}{c|c}
\hline
135^{k} & \downarrow & \downarrow \\
\hline
I & \downarrow & \downarrow \\$$



$$\Sigma F_{H} = -135 + 97.5 + \overline{EI}_{H}$$

$$\overline{EI}_{H} = \frac{37.5^{k}}{37.5^{k}} = \frac{37.5^{k}}{37.5^{2} + 37.5^{2}} = 53.03^{k} \text{ comp}$$

$$\Sigma F_{V} = 0 = \overline{FI}_{J} - 37.5 = 0$$

$$\overline{FI}_{J} = 37.5^{k} \text{ TEMS(on)}$$

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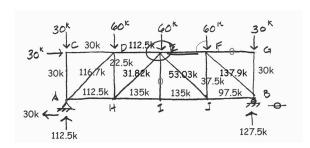
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Method of Joints - example

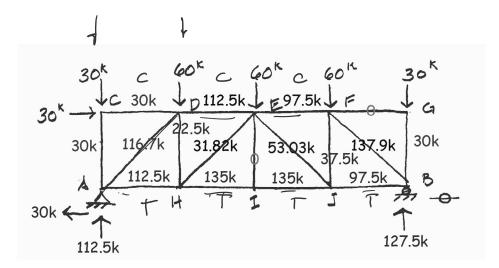
Solve the joints with the most members last.

Check that all forces balance.



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Inspect the final solution to see that it seems to make sense.



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Qualitative T or C

For typical gravity loading:

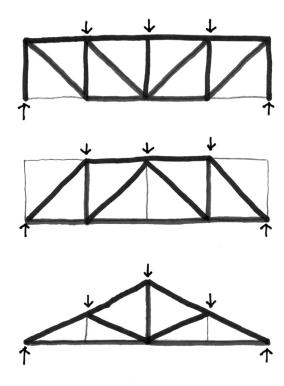
(tension=red compression=blue)

Top chords are in compression

Bottom chords are in tension

Diagonals down toward center are in tension (usually)

Diagonals up toward center are in compression (usually)

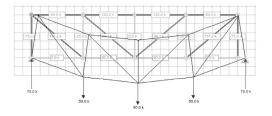


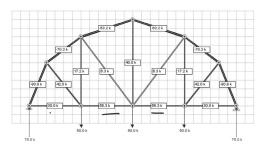
Qualitative Force

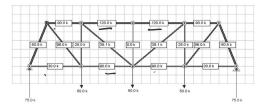
For spanning trusses with uniform loading: (tension=blue compression=red)

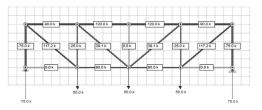
Top and bottom chords greatest at center when flat (at maximum curvature or moment)

Diagonals greatest at ends (near reactions, i.e. greatest shear)









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